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表 2.1 Green-Ampt 入滲參數

Soil class	Porosity η	Effective Porosity θ_e	Wetting front soil suction head $\Psi(\text{cm})$	Hydraulic conductivity $K(\text{cm/h})$
Sand	0.437 (0.374-0.500)	0.417 (0.354-0.480)	4.95 (0.97-25.36)	11.78
Loamy sand	0.437 (0.363-0.506)	0.401 (0.329-0.473)	6.13 (1.35-27.94)	2.99
Sandy loam	0.453 (0.351-0.555)	0.412 (0.283-0.541)	11.01 (2.67-45.47)	1.09
Loam	0.463 (0.375-0.551)	0.434 (0.334-0.534)	8.89 (1.33-59.38)	0.34
Silt loam	0.501 (0.420-0.582)	0.486 (0.394-0.578)	16.68 (2.92-95.39)	0.65
Sandy clay loam	0.398 (0.332-0.464)	0.330 (0.235-0.425)	21.85 (4.42-108.0)	0.15
Clay loam	0.464 (0.409-0.519)	0.309 (0.279-0.501)	20.88 (4.79-91.10)	0.10
Silty clay loam	0.471 (0.418-0.524)	0.432 (0.347-0.517)	27.30 (5.67-131.50)	0.10
Sandy clay	0.430 (0.370-0.490)	0.321 (0.207-0.435)	23.90 (4.08-140.2)	0.06
Silty clay	0.479 (0.425-0.533)	0.423 (0.334-0.512)	29.22 (6.13-139.4)	0.05
Clay	0.475 (0.427-0.523)	0.385 (0.269-0.501)	31.63 (6.39-156.5)	0.03

The numbers in parentheses below each parameter are one standard deviation around the parameter value given. (資料來源：Rawls, Brakensiek, and Miller, 1983.)

表 3.1 曼寧粗糙係數

Source	Ground Cover	<i>n</i>	Range
Crawford and Linsley(1966)	Smooth asphalt	0.012	
	Asphalt of concrete paving	0.014	
	Packed clay	0.03	
	Light turf	0.20	
	Dense turf	0.35	
	Dense shrubbery and forest litter	0.4	
Engman(1986)	Concrete or asphalt	0.011	0.01-0.13
	Bare sand	0.01	0.01-0.016
	Graveled surface	0.02	0.012-0.03
	Bare clay-loam(eroded)	0.02	0.012-0.033
	Range(natural)	0.13	0.01-0.32
	Bluegrass sod	0.45	0.39-0.63
	Short-grass prairie	0.15	0.10-0.20
	Bermuda grass	0.41	0.30-0.48

資料來源：Obtained by calibration of Stanford Watershed Model.

Computed by Engman(1986)by kinematic wave and storage analysis of measured rainfall-runoff data.

表 3.2 降雨強度表

Column	1	2	3	4
	Time (min)	Rainfall		
		Incremental (cm)	Cumulative (cm)	Intensity (cm/h)
	0		0.00	1.08
	10	0.18	0.18	1.26
	20	0.21	0.39	1.56
	30	0.26	0.65	1.92
	40	0.32	0.97	2.22
	50	0.37	1.34	2.58
	60	0.43	1.77	3.84
Ponding	70	0.64	2.41	6.84
	80	1.14	3.55	19.08
	90	3.18	6.73	9.90
	100	1.65	8.38	4.86
	110	0.81	9.19	3.12
	120	0.52	9.71	2.52
	130	0.42	10.13	2.16
	140	0.36	10.49	1.68
	150	0.28	10.77	1.44
	160	0.24	11.01	1.14
	170	0.19	11.20	1.02
	180	0.17	11.37	

(資料來源：Chow, Maidment, and Mays, 1988, Applied Hydrology)

表 4.1 下游 100 公尺處之最大單位寬度流量及發生時間

		均匀型	後退型	前進型	中央型
降雨延時 D=0.5 小時	K=0.34	0.00093 (1830s)	0.00092 (1405s)	0.00106 (1890s)	0.00102 (1720s)
	K=0.03	0.00122 (1800s)	0.00131 (1260s)	0.00156 (1880s)	0.00149 (1595s)
	K=0.0	0.00139 (1470s)	0.00165 (1145s)	0.00202 (1800s)	0.00186 (1485s)
降雨延時 D=1.0 小時	K=0.34	0.00041 (3600s)	0.00046 (2160s)	0.00072 (3640s)	0.00061 (2905s)
	K=0.03	0.00059 (2540s)	0.00076 (1750s)	0.00103 (3600s)	0.00094 (2560s)
	K=0.0	0.00069 (1980s)	0.00102 (1490s)	0.00116 (3600s)	0.00113 (2410s)
降雨延時 D=1.5 小時	K=0.34	0.00012 (7200s)	0.00014 (3985s)	0.00034 (7205s)	0.00024 (5320s)
	K=0.03	0.00027 (4405s)	0.00038 (2605s)	0.00053 (7200s)	0.00049 (4505s)
	K=0.0	0.00035 (2590s)	0.00057 (1955s)	0.00062 (7200s)	0.00061 (4295s)

註：最大單位寬度流量之單位為 m^3/s 。

表 4.2 坡地下游 400 公尺處之最大單位寬度流量及發生時間

		均匀型	後退型	前進型	中央型
降雨延時 D=0.5 小時	K=0.34	0.00093 (1755s)	0.00095 (1590s)	0.00106 (1800s)	0.00102 (1690s)
	K=0.03	0.00151 (1790s)	0.00152 (1710s)	0.00156 (1800s)	0.00154 (1720s)
	K=0.0	0.00214 (1900s)	0.00214 (1900s)	0.00214 (1900s)	0.00214 (1900s)

		均匀型	後退型	前進型	中央型
降雨延時 D=1.0 小時	K=0.34	0.00052 (3605s)	0.00055 (2925s)	0.00072 (3605s)	0.00066 (3300s)
	K=0.03	0.00127 (3605s)	0.00128 (3345s)	0.00135 (3605s)	0.00132 (3470s)
	K=0.0	0.00214 (3660s)	0.00214 (3660s)	0.00214 (3660s)	0.00214 (3660s)
降雨延時 D=1.5 小時	K=0.34	0.00013 (7205s)	0.00016 (4920s)	0.00035 (7200s)	0.00027 (6025s)
	K=0.03	0.00097 (7200s)	0.00096 (5705s)	0.00109 (7200s)	0.00105 (6720s)
	K=0.0	0.00139 (5675s)	0.00167 (4595s)	0.00202 (7200s)	0.00188 (5875s)

註：最大單位寬度流量之單位為 m^3/s 。

表 4.3 時變降雨之最終累積入滲深度表

		均匀型	後退型	前進型	中央型
降雨延時 D=0.5 小時	K=0.34	0.9890 (1800s)	0.8630 (1801s)	0.9690 (1774s)	0.9000 (1788s)
	K=0.03	0.4850 (1796s)	0.4460 (1795s)	0.4800 (1769s)	0.45800 (1777s)

		均匀型	後退型	前進型	中央型
降雨延時 D=1.0 小時	K=0.34	1.4367 (3601s)	1.2086 (3601s)	1.3997 (3599s)	1.2749 (3600s)
	K=0.03	0.6940 (3594s)	0.6240 (3593s)	0.6850 (3553s)	0.6460 (3575s)

		均匀型	後退型	前進型	中央型
降雨延時 D=1.5 小時	K=0.34	2.0471 (7201s)	1.6663 (7201s)	1.9923 (7197s)	1.7806 (7199s)
	K=0.03	0.9950 (7197s)	0.8700 (7198s)	0.9770 (7031s)	0.9080 (7138s)

註：累積入滲深度之單位為 cm。

表 4.4 最大單位寬度流量及發生時間(總降雨量不同且暴雨往下游移動)

		均匀型	後退型	前進型	中央型
$V_s=0.5\text{m/s}$ $D=1600\text{s}$	$K=0.34$	0.00073 (605s)	0.00129 (390s)	0.00153 (1590s)	0.00149 (915s)
	$K=0.03$	0.00082 (525s)	0.00143 (370s)	0.00162 (1590s)	0.00159 (915s)
	$K=0.0$	0.00083 (465s)	0.00151 (350s)	0.00164 (1590s)	0.00162 (910s)
$V_s=1.0\text{m/s}$ $D=800\text{s}$	$K=0.34$	0.00072 (535s)	0.00100 (390s)	0.00134 (800s)	0.00126 (545s)
	$K=0.03$	0.00081 (515s)	0.00115 (360s)	0.00145 (800s)	0.00138 (525s)
	$K=0.0$	0.00084 (465s)	0.00125 (350s)	0.00148 (800s)	0.00142 (520s)
$V_s=1.5\text{m/s}$ $D=534\text{s}$	$K=0.34$	0.00070 (530s)	0.00071 (385s)	0.00091 (535s)	0.00085 (475s)
	$K=0.03$	0.00080 (480s)	0.00087 (360s)	0.00117 (535s)	0.00103 (455s)
	$K=0.0$	0.00083 (450s)	0.00098 (350s)	0.00125 (530s)	0.00112 (450s)
$V_s=2.0\text{m/s}$ $D=400\text{s}$	$K=0.34$	0.00046 (420s)	0.00046 (360s)	0.00049 (420s)	0.00048 (405s)
	$K=0.03$	0.00062 (420s)	0.00060 (355s)	0.00063 (420s)	0.00062 (400s)
	$K=0.0$	0.00073 (420s)	0.00071 (350s)	0.00073 (420s)	0.00072 (400s)

註：最大單位寬度流量之單位為 m^3/s 。

表 4.5 最大單位寬度流量及發生時間(總降雨量不同且暴雨往上游移動)

		均匀型	前進型	後退型	中央型
$V_s=0.5\text{m/s}$ $D=1600\text{s}$	K=0.34	0.00073 (800s)	0.00114 (560s)	0.00127 (1400s)	0.00124 (945s)
	K=0.03	0.00081 (760s)	0.00124 (550s)	0.00137 (1400s)	0.00134 (935s)
	K=0.0	0.00083 (660s)	0.00128 (540s)	0.00139 (1400s)	0.00136 (925s)
$V_s=1.0\text{m/s}$ $D=800\text{s}$	K=0.34	0.00071 (600s)	0.00086 (465s)	0.00098 (705s)	0.00094 (605s)
	K=0.03	0.00080 (575s)	0.00097 (450s)	0.00114 (700s)	0.00107 (585s)
	K=0.0	0.00083 (555s)	0.00102 (445s)	0.00118 (700s)	0.00112 (570s)
$V_s=1.5\text{m/s}$ $D=534\text{s}$	K=0.34	0.00056 (520s)	0.00057 (450s)	0.00057 (535s)	0.00057 (515s)
	K=0.03	0.00070 (485s)	0.00070 (430s)	0.00070 (520s)	0.00070 (510s)
	K=0.0	0.00076 (470s)	0.00075 (420s)	0.00077 (470s)	0.00076 (470s)
$V_s=2.0\text{m/s}$ $D=400\text{s}$	K=0.34	0.00037 (415s)	0.00037 (410s)	0.00037 (415s)	0.00037 (410s)
	K=0.03	0.00046 (415s)	0.00046 (400s)	0.00046 (415s)	0.00046 (405s)
	K=0.0	0.00051 (415s)	0.00051 (400s)	0.00051 (410s)	0.00051 (405s)

註：最大單位寬度流量之單位為 m^3/s 。

表 4.6 最大單位寬度流量及發生時間(總降雨量相同且暴雨往下游移動)

		均匀型	前進型	後退型	中央型
$V_s=0.3\text{m/s}$	$K=0.34$	0.00079 (605s)	0.00121 (410s)	0.00146 (965s)	0.00146 (650s)
	$K=0.03$	0.00081 (545s)	0.00137 (390s)	0.00155 (960s)	0.00155 (645s)
	$K=0.0$	0.00084 (505s)	0.00146 (380s)	0.00158 (950s)	0.00158 (635s)
$V_s=0.5\text{m/s}$	$K=0.34$	0.00072 (550s)	0.00109 (395s)	0.00148 (895s)	0.00140 (570s)
	$K=0.03$	0.00081 (530s)	0.00125 (365s)	0.00158 (895s)	0.00151 (560s)
	$K=0.0$	0.00083 (480s)	0.00136 (345s)	0.00160 (890s)	0.00155 (555s)
$V_s=1.0\text{m/s}$	$K=0.34$	0.00072 (540s)	0.00100 (385s)	0.00134 (800s)	0.00126 (545s)
	$K=0.03$	0.00081 (515s)	0.00115 (360s)	0.00145 (800s)	0.00138 (525s)
	$K=0.0$	0.00083 (480s)	0.00125 (345s)	0.00148 (800s)	0.00142 (515s)
$V_s=2.0\text{m/s}$	$K=0.34$	0.00071 (520s)	0.00097 (385s)	0.00126 (745s)	0.00118 (525s)
	$K=0.03$	0.00081 (490s)	0.00111 (360s)	0.00137 (745s)	0.00130 (505s)
	$K=0.0$	0.00083 (480s)	0.00119 (345s)	0.00140 (745s)	0.00135 (500s)

註：最大單位寬度流量之單位為 m^3/s 。

表 4.7 最大單位寬度流量及發生時間(總降雨量相同且暴雨往上游移動)

		均匀型	前進型	後退型	中央型
$V_s=0.3\text{m/s}$	$K=0.34$	0.00060 (770s)	0.00061 (705s)	0.00061 (805s)	0.00061 (745s)
	$K=0.03$	0.00071 (745s)	0.00071 (700s)	0.00071 (755s)	0.00071 (725s)
	$K=0.0$	0.00075 (715s)	0.00075 (680s)	0.00075 (720s)	0.00075 (710s)
$V_s=0.5\text{m/s}$	$K=0.34$	0.00071 (700s)	0.00075 (565s)	0.00077 (795s)	0.00077 (690s)
	$K=0.03$	0.00080 (660s)	0.00086 (555s)	0.00091 (750s)	0.00091 (670s)
	$K=0.0$	0.00083 (640s)	0.00090 (550s)	0.00097 (700s)	0.00096 (660s)
$V_s=1.0\text{m/s}$	$K=0.34$	0.00071 (600s)	0.00086 (465s)	0.00098 (705s)	0.00094 (610s)
	$K=0.03$	0.00080 (580s)	0.00097 (455s)	0.00114 (700s)	0.00107 (585s)
	$K=0.0$	0.00083 (565s)	0.00102 (450s)	0.00118 (700s)	0.00112 (575s)
$V_s=2.0\text{m/s}$	$K=0.34$	0.00071 (550s)	0.00091 (420s)	0.00111 (700s)	0.00102 (565s)
	$K=0.03$	0.00080 (530s)	0.00102 (405s)	0.00122 (700s)	0.00115 (550s)
	$K=0.0$	0.00083 (510s)	0.00108 (400s)	0.00126 (700s)	0.00120 (535s)

註：最大單位寬度流量之單位為 m^3/s 。